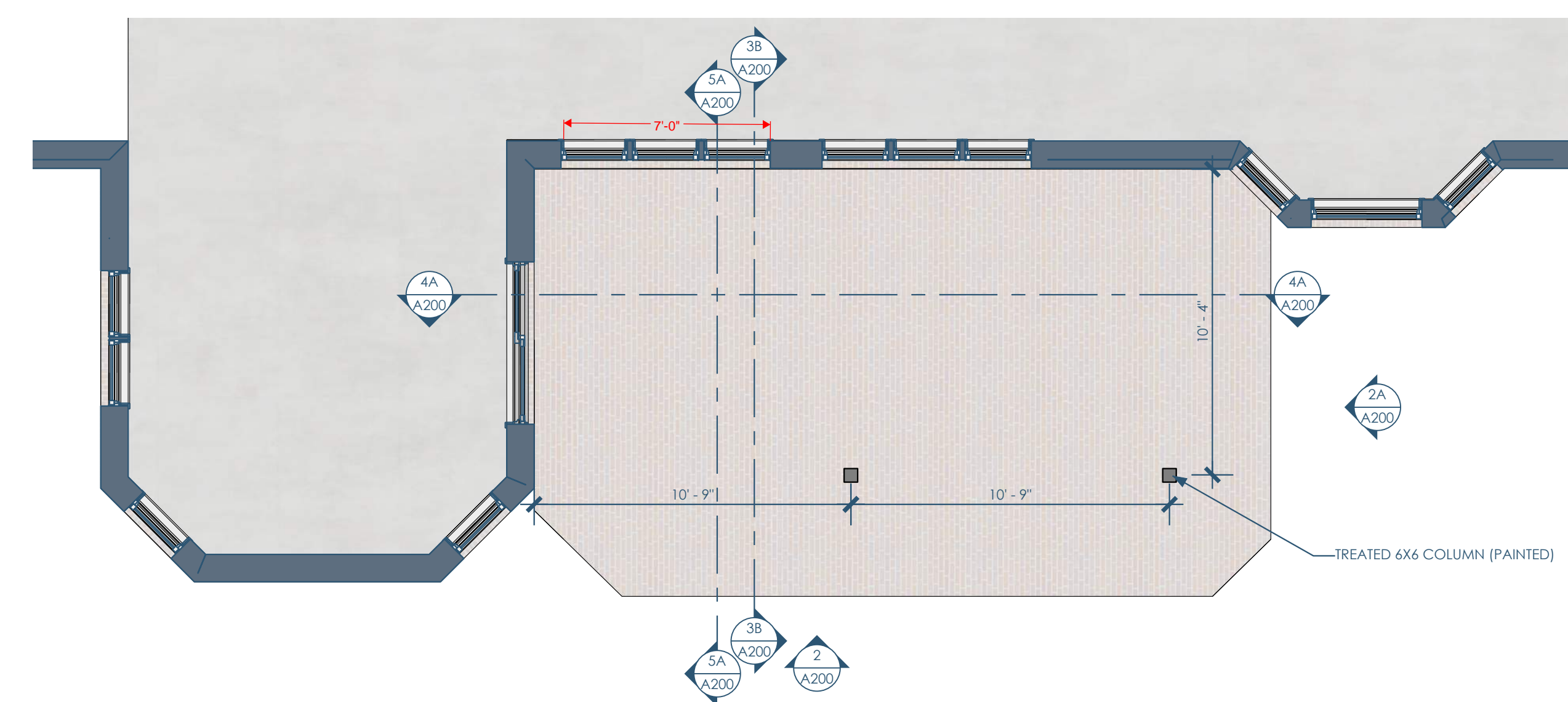


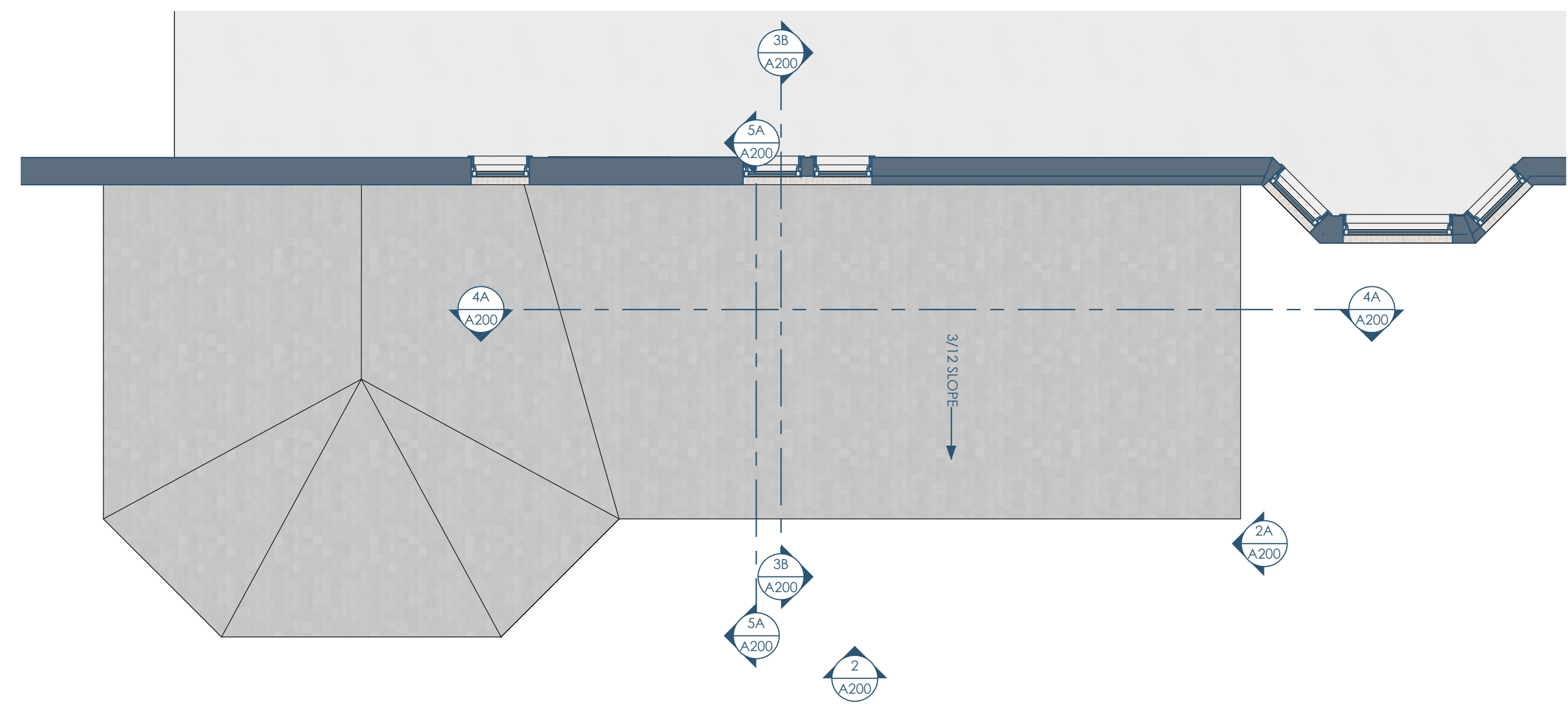
5C/A100 FLOOR PLAN - 1ST FLOOR
1/4"=1'-0"

proposing to use
4x4's, between
screens **THAT WORKS**

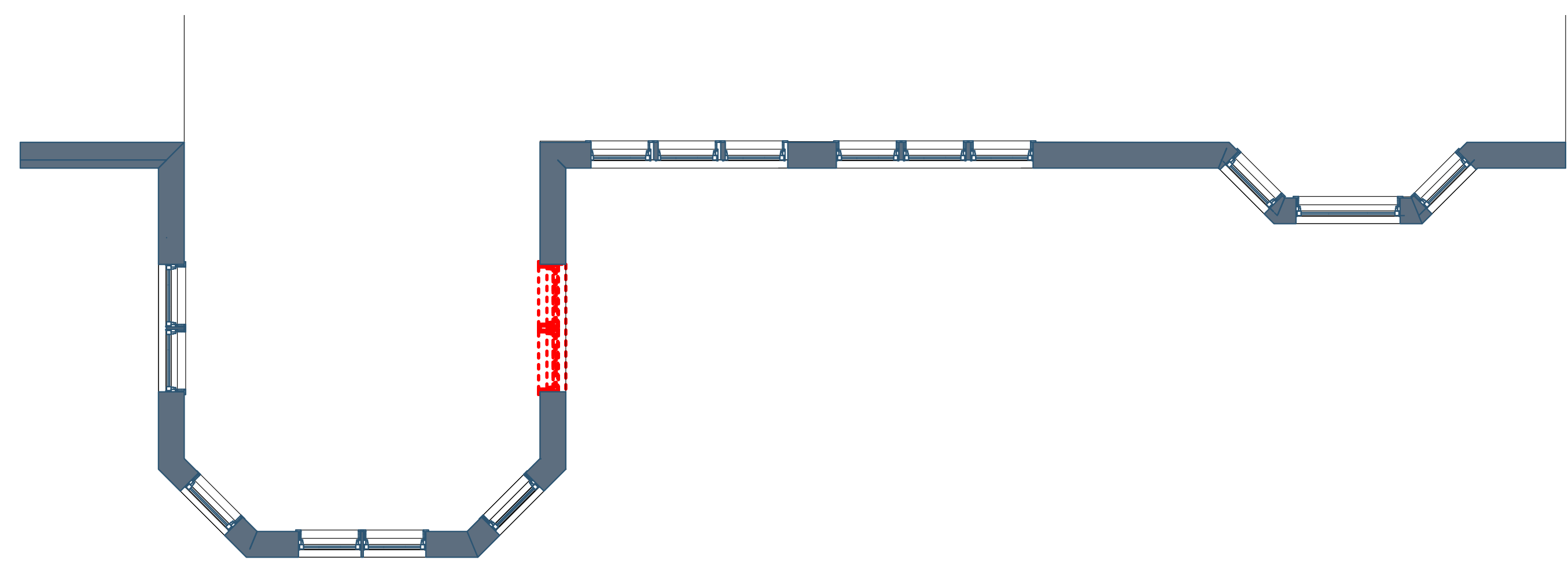
will be screens only **OK**



3C/A100 FLOOR PLAN - BASEMENT
1/4"=1'-0"

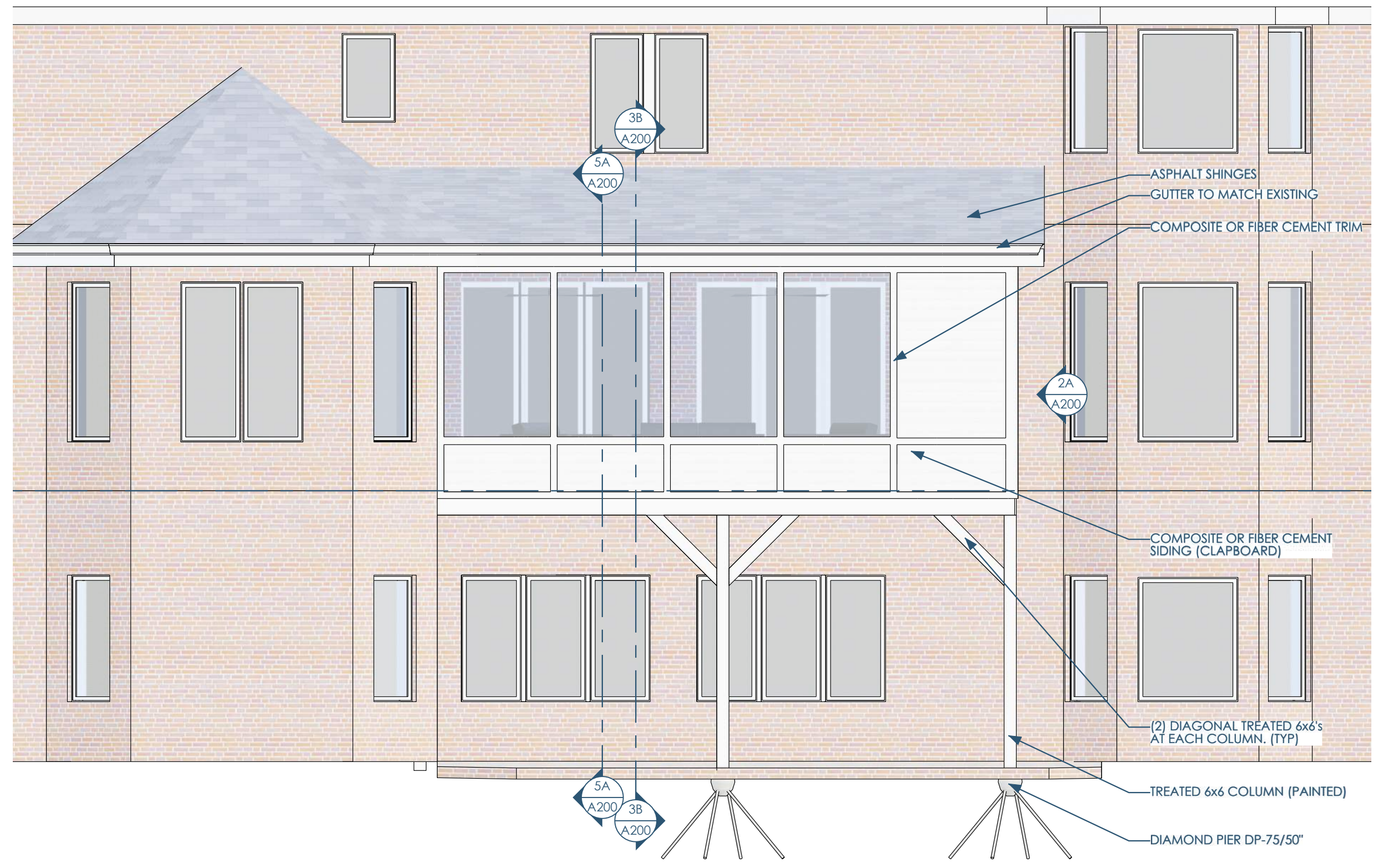


5A/A100 FLOOR PLAN - ROOF
1/4"=1'-0"

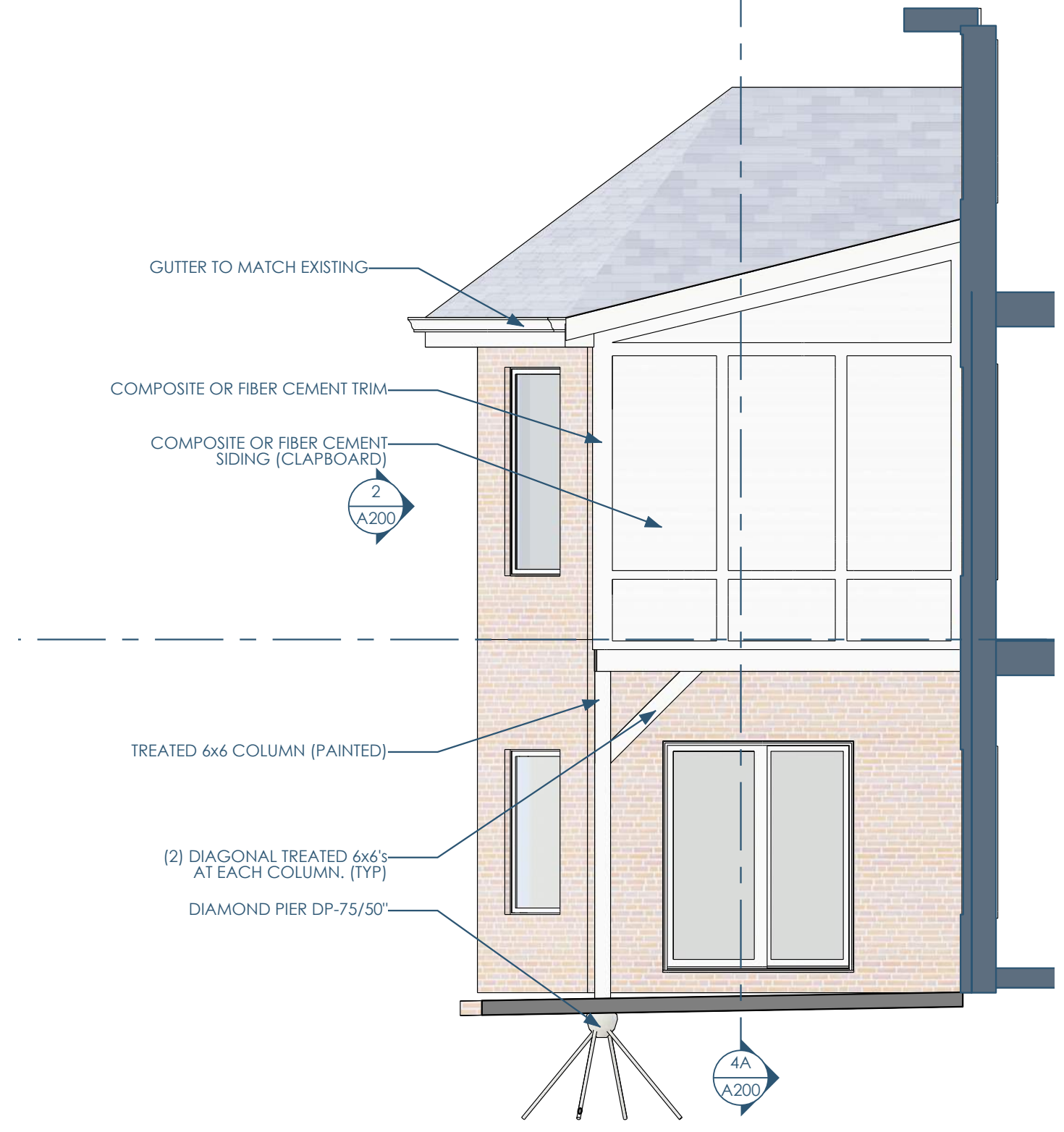


3A/A100 DEMO PLAN
1/4"=1'-0"

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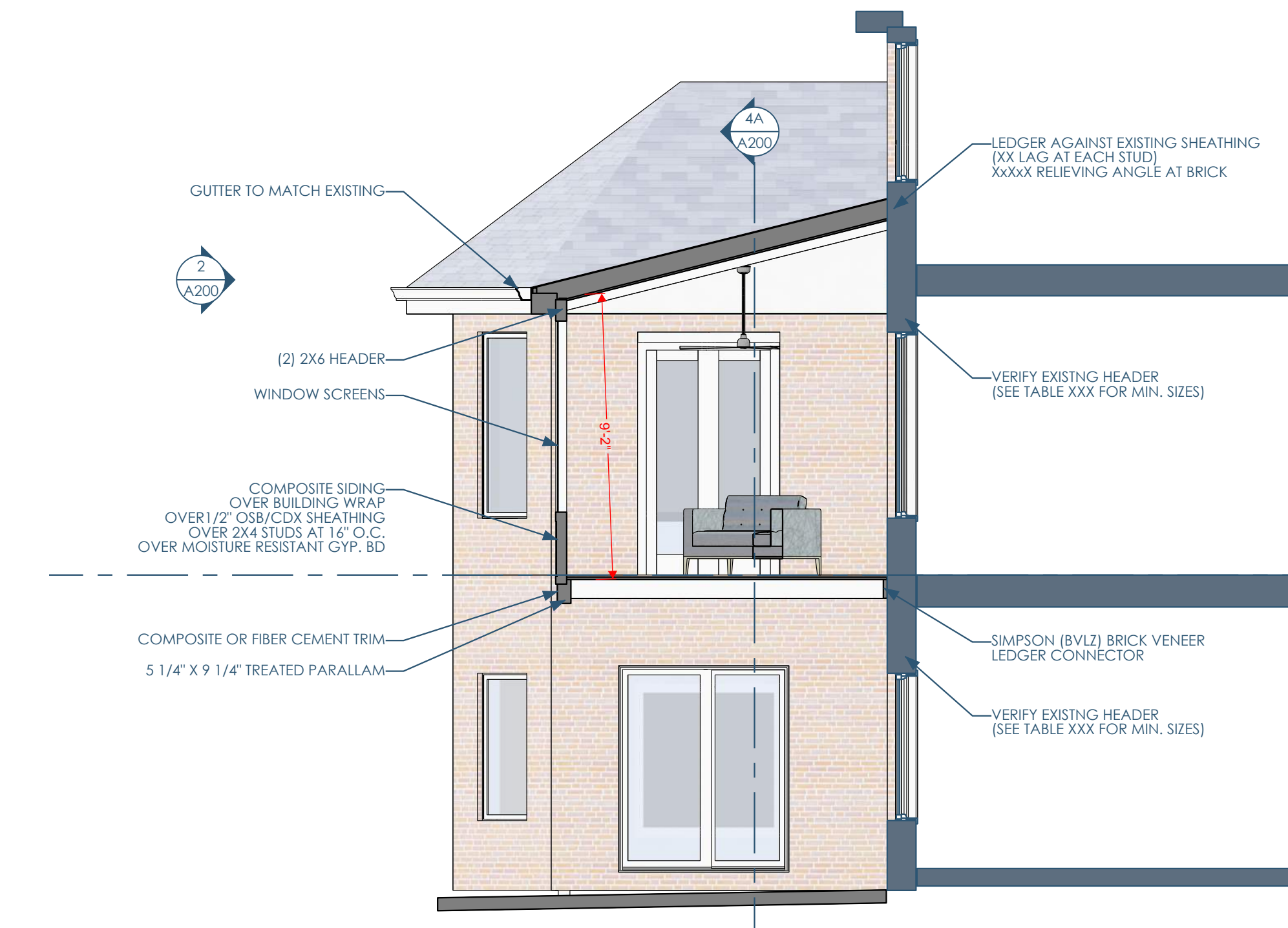
2/A200 ELEVATION - NORTH
1/4"=1'-0"



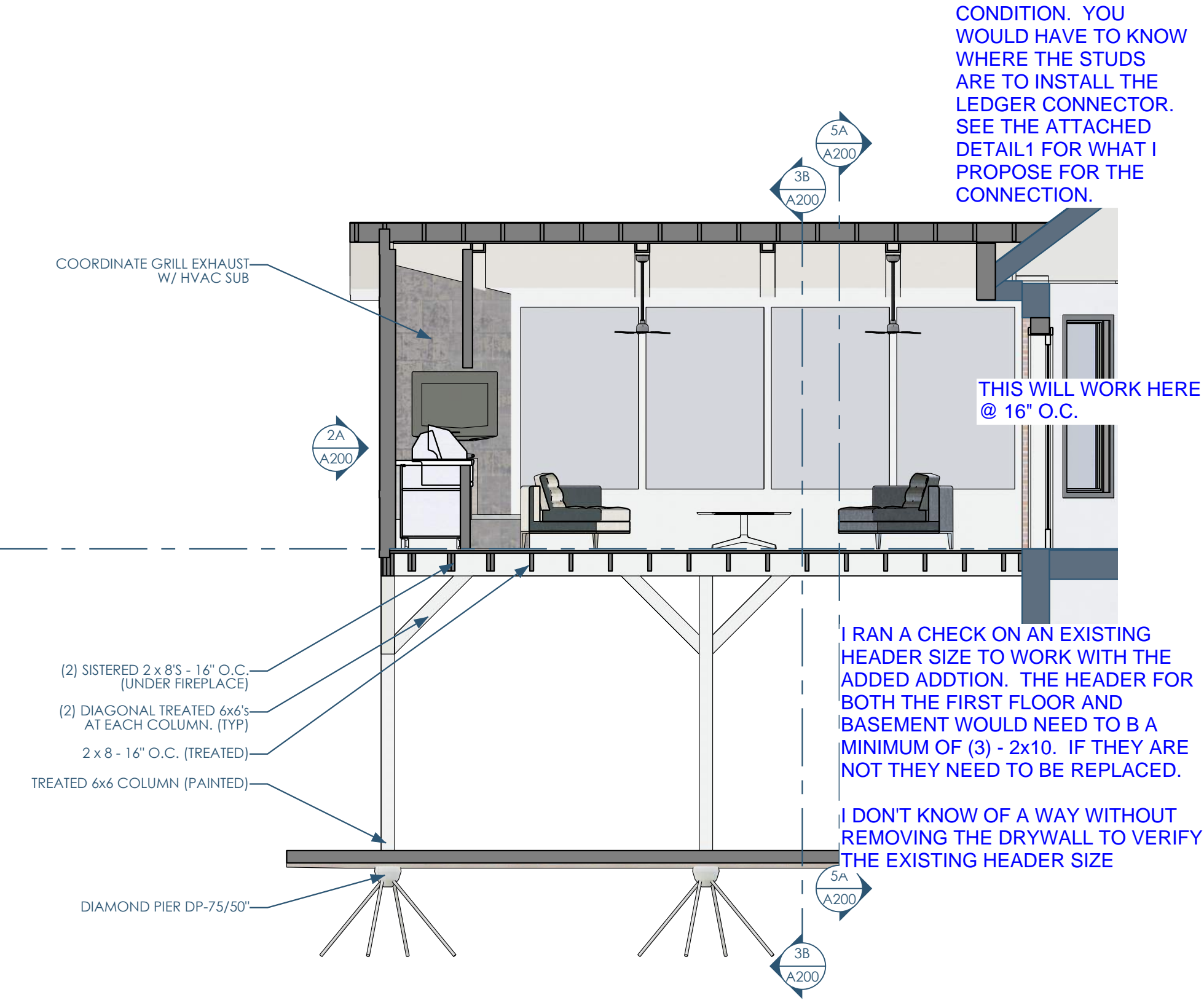
2A/A200 ELEVATION - EAST
1/4"=1'-0"

In general need guidance on overall lateral stability of the proposed design.

THE LATERAL STABILITY IS FINE AS LONG AS WE ARE TIED INTO THE EXISTING HOUSE.



5A/A200 SECTION - EAST-WEST - LOOKING SOUTH
1/4"=1'-0"



4A/A200 SECTION - NORTH-SOUTH - LOOKING EAST
1/4"=1'-0"

THE BVLZ WILL NOT WORK FOR THIS CONDITION. YOU WOULD HAVE TO KNOW WHERE THE STUDS ARE TO INSTALL THE LEDGER CONNECTOR. SEE THE ATTACHED DETAIL1 FOR WHAT I PROPOSE FOR THE CONNECTION.

THIS WILL WORK HERE @ 16\"/>

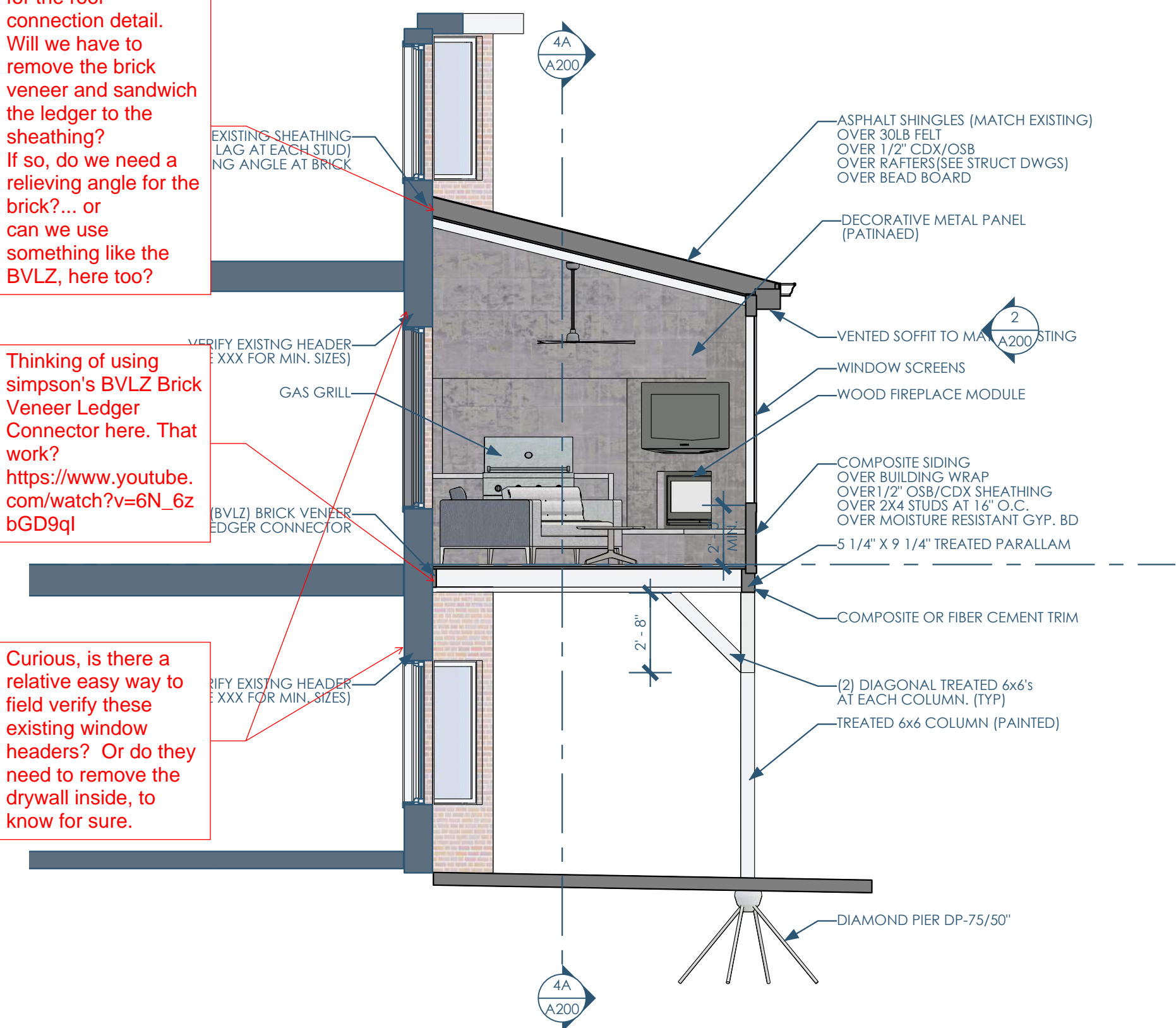
I RAN A CHECK ON AN EXISTING HEADER SIZE TO WORK WITH THE ADDED ADDITION. THE HEADER FOR BOTH THE FIRST FLOOR AND BASEMENT WOULD NEED TO BE A MINIMUM OF (3) - 2x10. IF THEY ARE NOT THEY NEED TO BE REPLACED.

I DON'T KNOW OF A WAY WITHOUT REMOVING THE DRYWALL TO VERIFY THE EXISTING HEADER SIZE

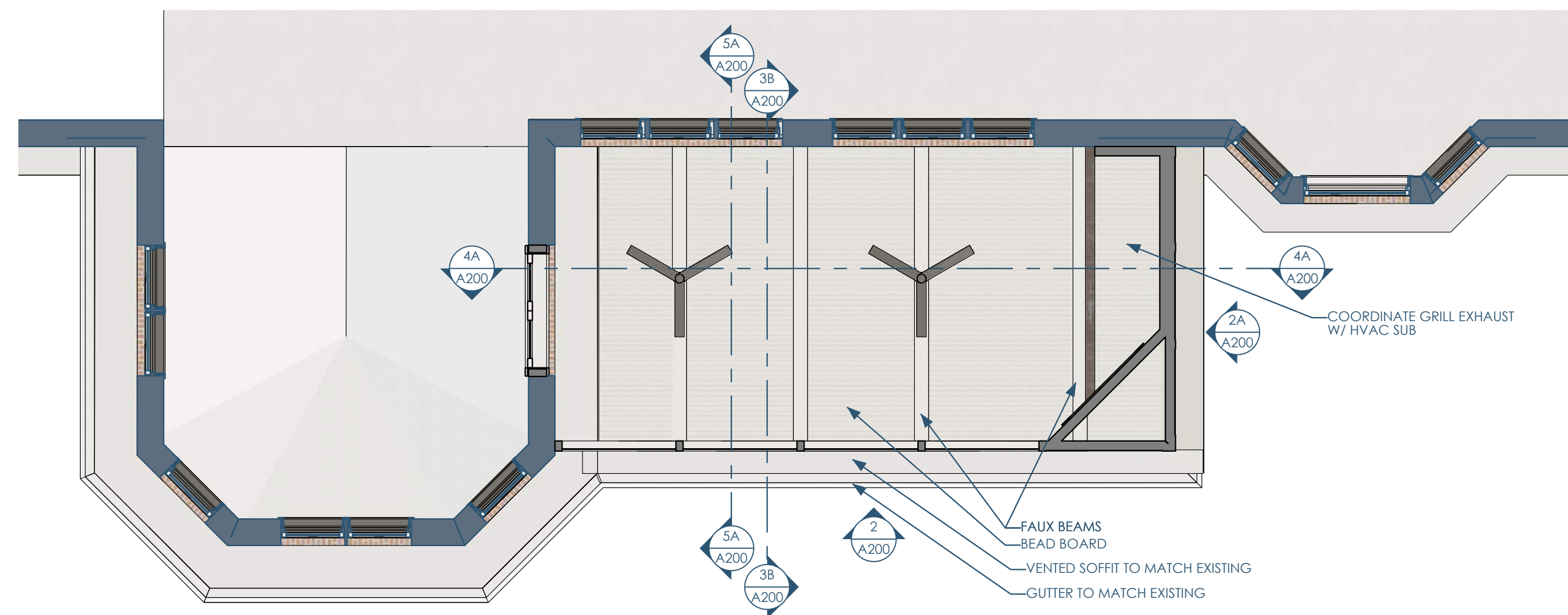
Looking for guidance for the roof connection detail. Will we have to remove the brick veneer and sandwich the ledger to the sheathing? If so, do we need a relieving angle for the brick?... or can we use something like the BVLZ, here too?

Thinking of using simpson's BVLZ Brick Veneer Ledger Connector here. That work? https://www.youtube.com/watch?v=6N_6z bGD9qI

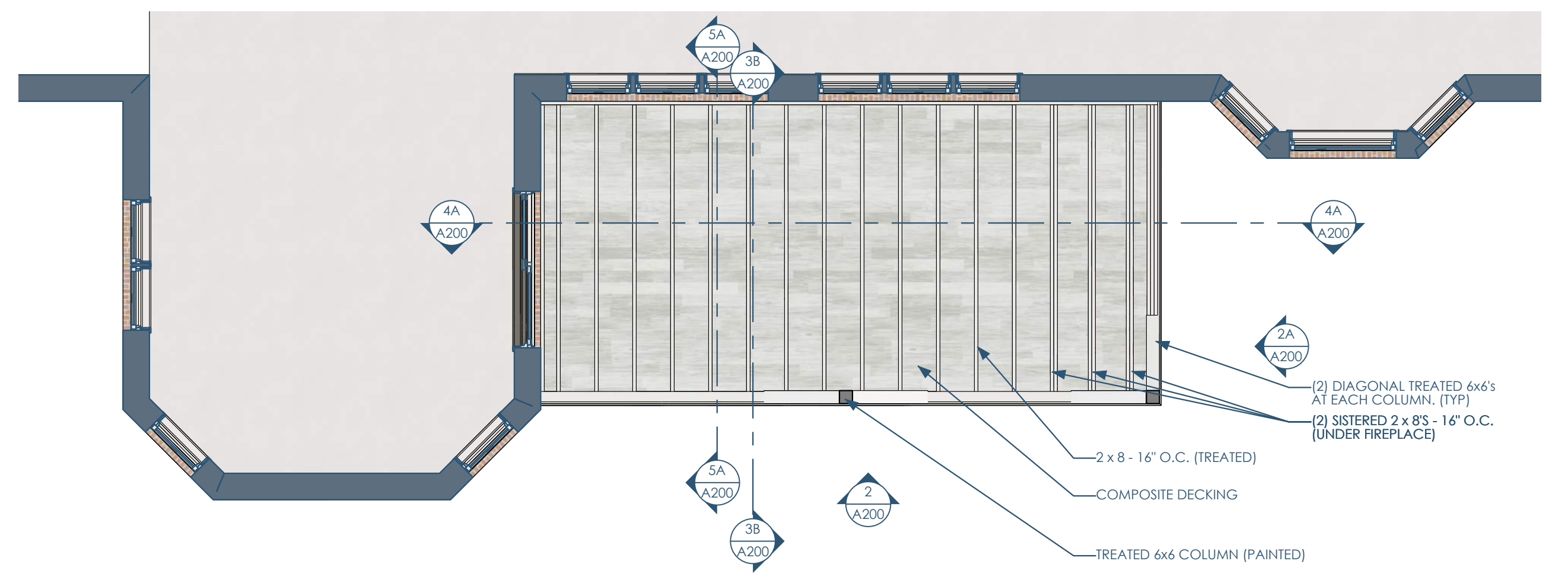
Curious, is there a relative easy way to field verify these existing window headers? Or do they need to remove the drywall inside, to know for sure.



3B/A200 SECTION - EAST-WEST - LOOKING NORTH
1/4"=1'-0"



5C/A150 RCP - 1ST FLOOR
1/4"=1'-0"



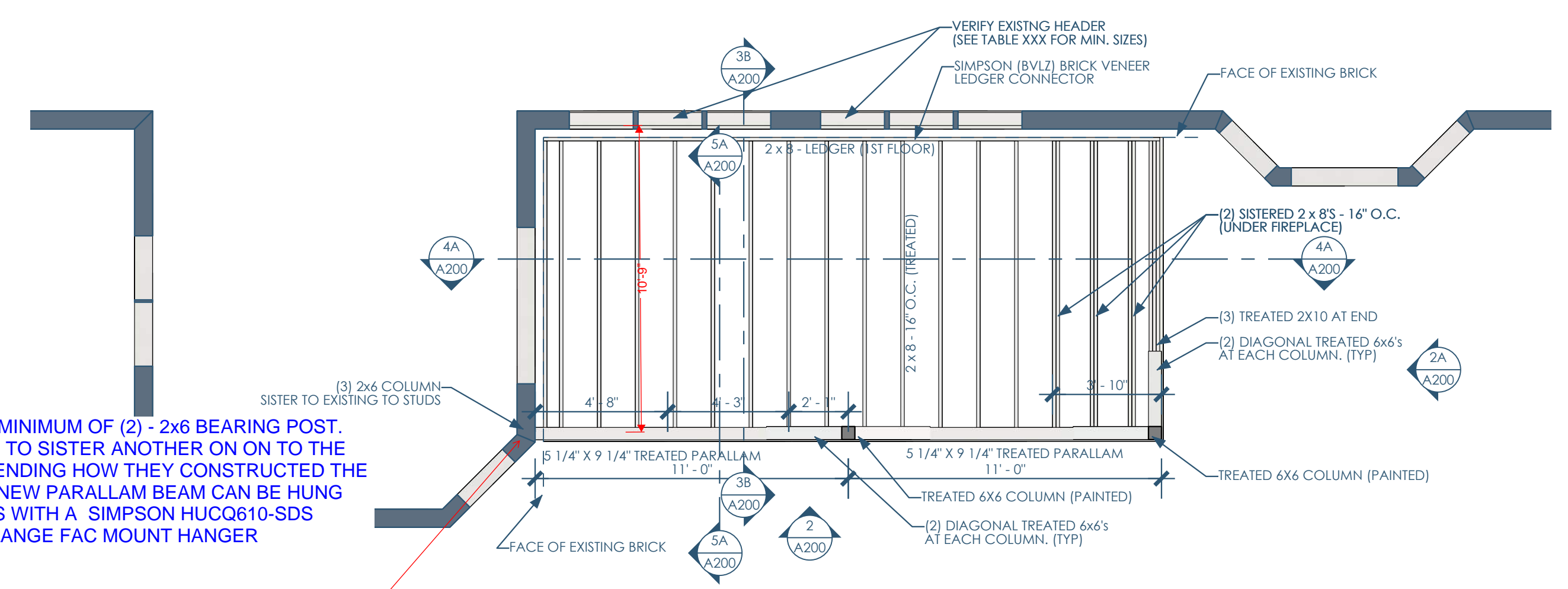
5B/A150 RCP - BASEMENT
1/4"=1'-0"

THIS CAN BE A MINIMUM OF (2) - 2x6 BEARING POST. YOU MAY NEED TO SISTER ANOTHER ON ON TO THE EXISTING, DEPENDING HOW THEY CONSTRUCTED THE CORNER. THE NEW PARALLAM BEAM CAN BE HUNG OFF THE STUDS WITH A SIMPSON HUCQ610-SDS CONCEALED FLANGE FAC MOUNT HANGER

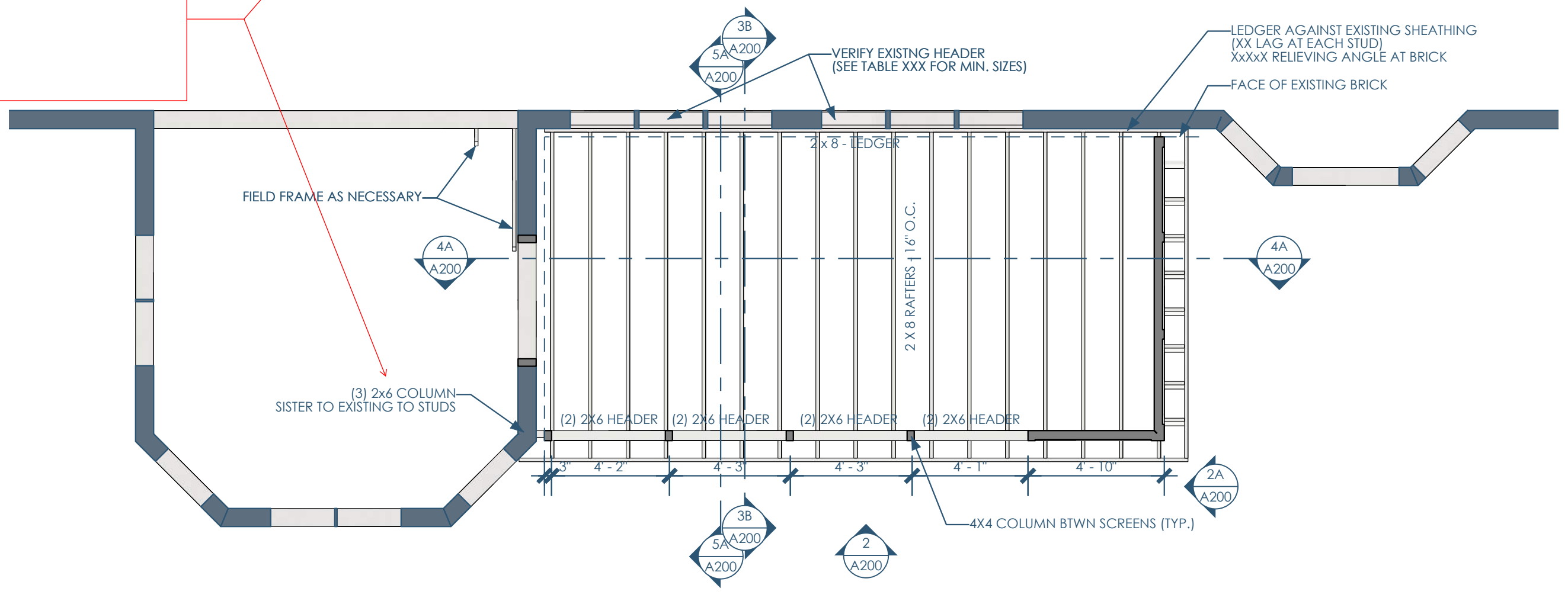
what do you suggest for this column? Obviously there's existing studs there. What's the min. this should be?

Projected loads...
2nd floor: 621lbs
1st Floor Column: 2592 lbs.

I GET MORE OF A LOAD OF 4600 LBS. THERE IS A BIT OF SNOW DRIFT AND SLIDING THAT NEEDS TO BE ACCOUNTED FOR ON THE ROOF.



3/A150 FRAMING PLAN - 1ST FLOOR
1/4"=1'-0"



4/A150 FRAMING PLAN - ROOF
1/4"=1'-0"



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Madison, WI 53703
ryan@openingdesign.com | 773.425.6456

GC: Lake Country Construction Group
161 Horizon Dr
Verona, WI 53593
608.845.1398

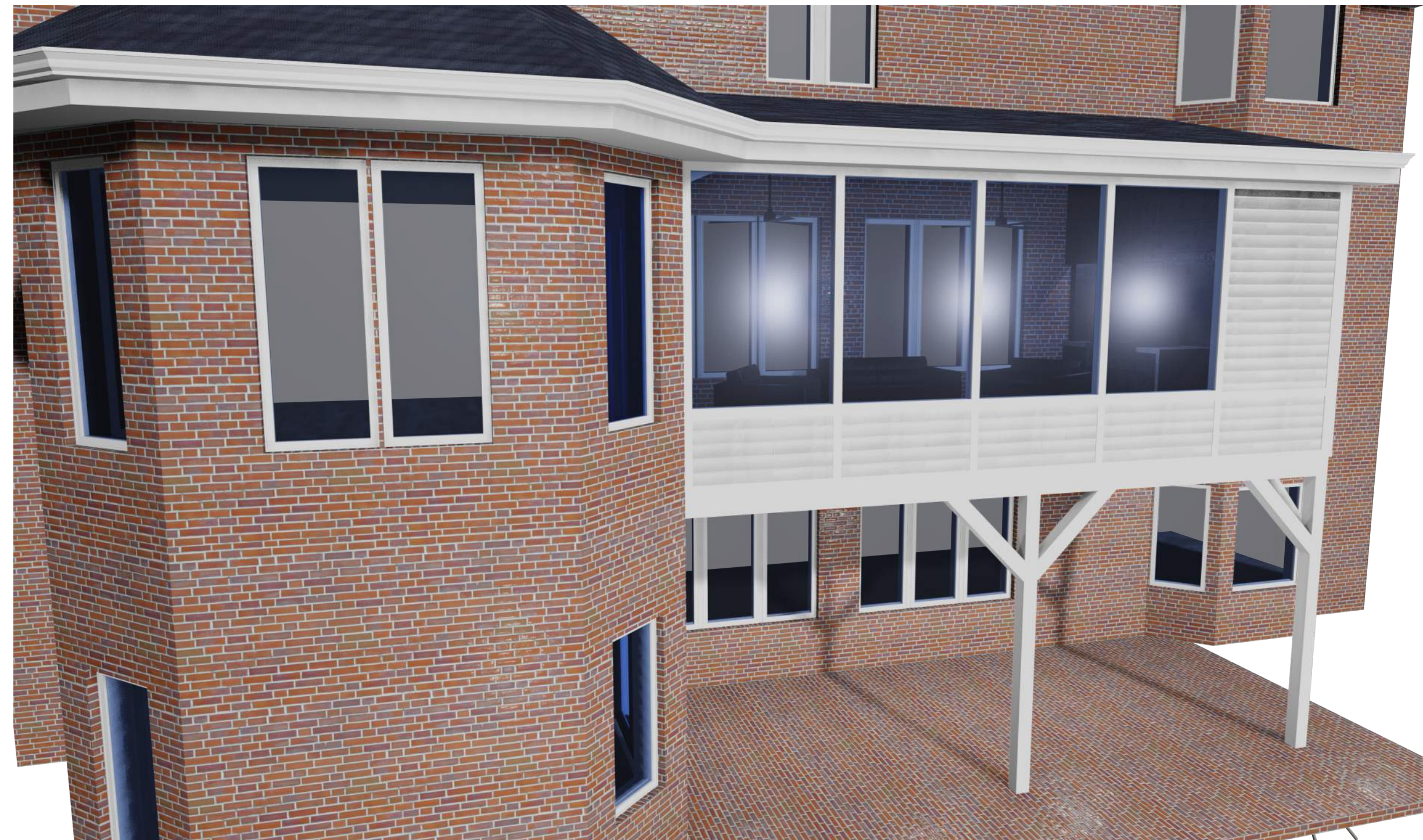
REFLECTED CEILING PLANS
HERNAN 3 SEASON PORCH
VERONA, WI

A150

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4/A900 PERSPECTIVE 1
3/16"=1'-0"



3B/A900 PERSPECTIVE 2
3/16"=1'-0"



5A/A900 PERSPECTIVE 3
3/16"=1'-0"



3A/A900 PERSPECTIVE 4
3/16"=1'-0"



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RENDERINGS
HERNAN 3 SEASON PORCH
VERONA, WI

A900

In general, can you verify if the following calcs
look correct?

I also have a specific question on the
basement window header, below, as well.

Level			
Member Name	Results (Max UTIL %)	Current Solution	Comments
Middle Column	Passed (56% B/C)	1 piece(s) 6 x 6 SP No.2	
Roof Beam - (2) 2X6 HEADER	Passed (45% M)	2 piece(s) 2 x 6 SP No.2	
1st Floor without fireplace - 2 x 8 - 16" O.C. (TREATED)	Passed (93% M)	1 piece(s) 2 x 8 SP No.2 @ 16" OC	
1st Floor w/ Fireplace - (2) SISTERED 2 x 8'S - 16" O.C.	Passed (73% M)	2 piece(s) 2 x 8 SP No.2 @ 16" OC	
Beam with Fireplace - 5 1/4" x 9 1/4" TREATED PARALLAM	Passed (91% R)	1 piece(s) 5 1/4" x 9 1/4" Treated Parallam® Plus PSL SL1 - Damp Use (12% < MC <= 16%)	
Beam (not fireplace) - 5 1/4" x 9 1/4" TREATED PARALLAM	Passed (57% ΔT)	1 piece(s) 5 1/4" x 9 1/4" Treated Parallam® Plus PSL SL1 - Damp Use (12% < MC <= 16%)	
Existing Window Header - Basement - 7ft	Passed (97% R)	3 piece(s) 1 3/4" x 9 1/2" 1.55E TimberStrand® LSL	
Existing Window Header - First Floor - 7ft	Passed (93% R)	2 piece(s) 1 1/2" x 9 1/2" 1.5E TimberStrand® LSL	

FortewEB Software Operator	Job Notes
Ryan Schultz OpeningDesign (773) 425-6456 ryan@openingdesign.com	



11/8/2024 6:33:56 PM UTC

ForteWEB v3.8

File Name: Project

Level, Middle Column

1 piece(s) 6 x 6 SP No.2

Post Height: 10'



Design Results	Actual	Allowed	Result	LDF	Load: Combination
Slenderness	22	50	Passed (44%)	--	--
Compression (lbs)	6000	12725	Passed (47%)	1.00	1.0 D + 1.0 L
Base Bearing (lbs)	6000	898425	Passed (1%)	--	1.0 D + 1.0 L
Bending/Compression	0.56	1	Passed (56%)	1.00	1.0 D + 1.0 L

- Input axial load eccentricity for this design is 16.67% of applicable member side dimension.
- Applicable calculations are based on NDS.

Supports	Type	Material
Base	Plate	Steel

Member Type : Free Standing Post
 Building Code : IBC 2015
 Design Methodology : ASD

Max Unbraced Length	Comments
Full Member Length	No bracing assumed.

Drawing is Conceptual

Vertical Load	Dead (0.90)	Floor Live (1.00)	Comments
1 - Point (lb)	1000	5000	Default Load

Weyerhaeuser Notes

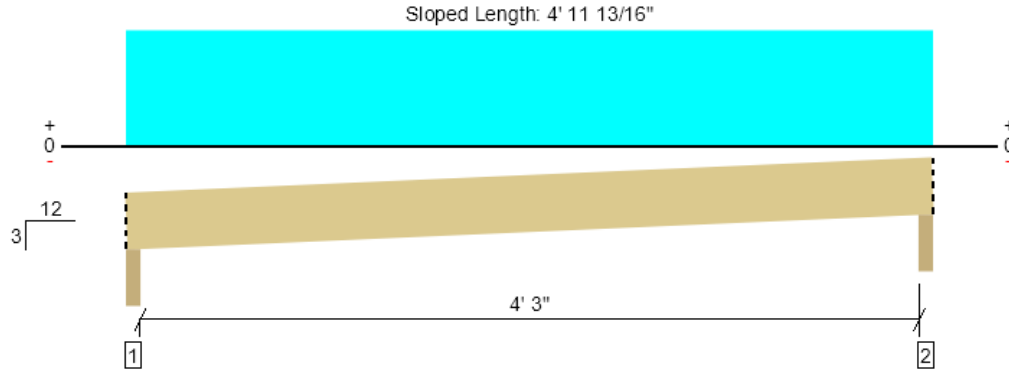
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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
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Level, Roof Beam - (2) 2X6 HEADER
2 piece(s) 2 x 6 SP No.2



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	621 @ 2"	4463 (3.50")	Passed (14%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	432 @ 8 13/16"	2214	Passed (20%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	650 @ 2' 5"	1449	Passed (45%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.028 @ 2' 5"	0.232	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.043 @ 2' 5"	0.309	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

Member Length : 5' 1 3/16"
 System : Roof
 Member Type : Drop Beam
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD
 Member Pitch : 3/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Beveled Plate - SPF	3.50"	3.50"	1.50"	222	399	621	Blocking
2 - Beveled Plate - SPF	3.50"	3.50"	1.50"	222	399	621	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' o/c	
Bottom Edge (Lu)	5' o/c	

- Maximum allowable bracing intervals based on applied load.
- Dimensions for lateral bracing intervals are measured along the length of the member for sloped conditions.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 4' 10"	N/A	4.2	--	
1 - Uniform (PSF)	0 to 4' 10"	5' 6"	15.5	30.0	Roof Load

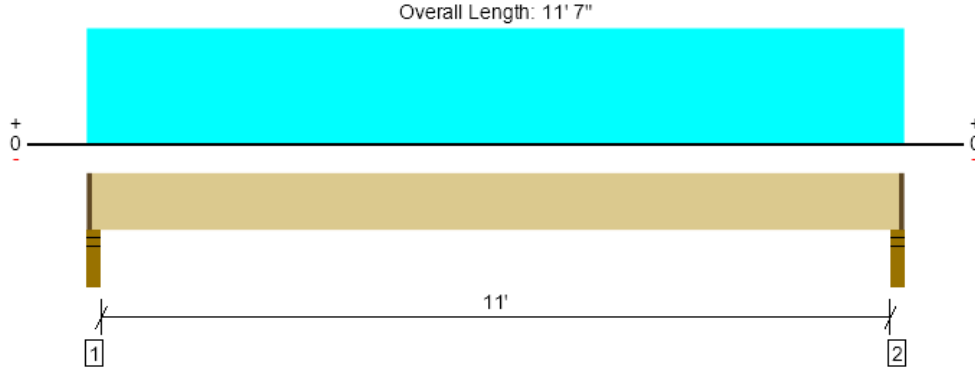
• Side loads are assumed to not induce cross-grain tension.

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ForteWEB Software Operator	Job Notes
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Level, 1st Floor without fireplace - 2 x 8 - 16" O.C. (TREATED)
1 piece(s) 2 x 8 SP No.2 @ 16" OC



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	394 @ 2 1/2"	1434 (2.25")	Passed (27%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	339 @ 10 3/4"	1269	Passed (27%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1081 @ 5' 9 1/2"	1165	Passed (93%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.280 @ 5' 9 1/2"	0.372	Passed (L/479)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.364 @ 5' 9 1/2"	0.558	Passed (L/368)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	N/A	N/A	N/A	--	N/A

Member Length : 11' 4 1/2"
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3.50"	2.25"	1.50"	93	309	402	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	1.50"	93	309	402	1 1/4" Rim Board

• Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 10" o/c	
Bottom Edge (Lu)	11' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 11' 7"	16"	12.0	40.0	Floor Load

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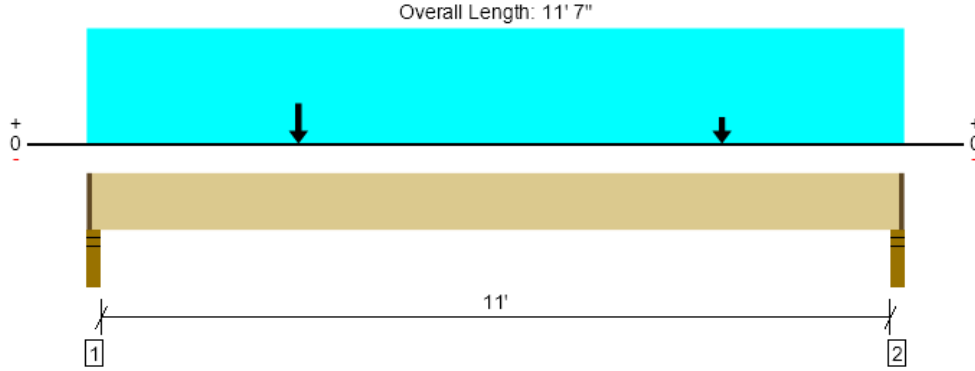
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
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11/8/2024 6:33:56 PM UTC
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 File Name: Project

Level, 1st Floor w/ Fireplace - (2) SISTERED 2 x 8'S - 16" O.C.
2 piece(s) 2 x 8 SP No.2 @ 16" OC



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	651 @ 2 1/2"	2869 (2.25")	Passed (23%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	596 @ 10 3/4"	2538	Passed (24%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1691 @ 5' 2 1/16"	2329	Passed (73%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.251 @ 5' 8 1/4"	0.372	Passed (L/533)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.293 @ 5' 8 3/8"	0.558	Passed (L/457)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	N/A	N/A	N/A	--	N/A

Member Length : 11' 4 1/2"
 System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3.50"	2.25"	1.50"	93	566	658	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	1.50"	93	502	595	1 1/4" Rim Board

• Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 5" o/c	
Bottom Edge (Lu)	11' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 11' 7"	16"	12.0	40.0	Floor Load
2 - Point (lb)	3'	N/A	-	300	Fireplace
3 - Point (lb)	9'	N/A	-	150	Gas Grill

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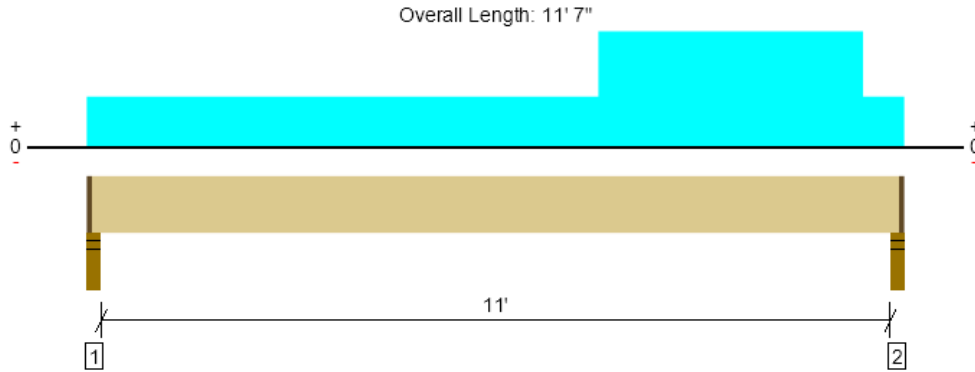
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
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11/8/2024 6:33:56 PM UTC
 ForteWEB v3.8, Engine: V8.4.1.24, Data: V8.1.6.3
 File Name: Project

Level, Beam with Fireplace - 5 1/4" x 9 1/4" TREATED PARALLAM
1 piece(s) 5 1/4" x 9 1/4" Treated Parallam® Plus PSL SL1 - Damp Use (12% < MC <= 16%)



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4108 @ 11' 5"	4518 (2.25")	Passed (91%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	3431 @ 10' 6 1/4"	7042	Passed (49%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	9093 @ 7' 2 1/16"	14526	Passed (63%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.292 @ 6'	0.375	Passed (L/462)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.428 @ 5' 11 7/16"	0.563	Passed (L/315)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 11' 4 1/2"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Member capacities and stiffness have been reduced to account for user specified Service Level and/or environmental treatment.
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - SPF	3.50"	2.25"	1.50"	879	1777	956	2928	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	2.05"	905	3239	956	4144	1 1/4" Rim Board

• Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 5" o/c	
Bottom Edge (Lu)	11' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	1 1/4" to 11' 5 3/4"	N/A	16.4	--	--	
1 - Uniform (PSF)	0 to 11' 7" (Front)	5' 6"	12.0	40.0	-	Floor
2 - Uniform (PSF)	0 to 11' 7" (Front)	5' 6"	12.4	-	30.0	Roof
3 - Uniform (PLF)	7' 3" to 11' (Front)	N/A	12.0	658.0	-	Additional Loads from Fireplace and Gas Grill

• Side loads are assumed to not induce cross-grain tension.

Weyerhaeuser Notes

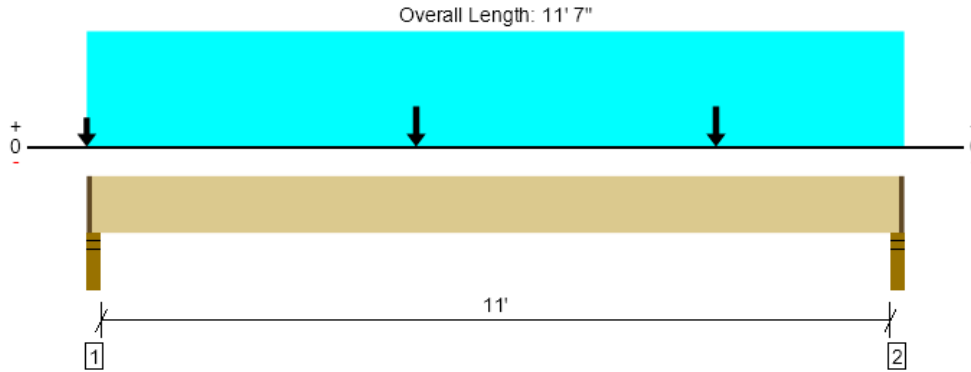
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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Ryan Schultz OpeningDesign (773) 425-6456 ryan@openingdesign.com	



Level, Beam (not fireplace) - 5 1/4" x 9 1/4" TREATED PARALLAM
1 piece(s) 5 1/4" x 9 1/4" Treated Parallam® Plus PSL SL1 - Damp Use (12% < MC <= 16%)



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2402 @ 11' 5"	4518 (2.25")	Passed (53%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	2165 @ 10' 6 1/4"	8098	Passed (27%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	6917 @ 5' 2 3/16"	16705	Passed (41%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.199 @ 5' 9 13/16"	0.375	Passed (L/680)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.320 @ 5' 9 13/16"	0.563	Passed (L/422)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 11' 4 1/2"
 System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Member capacities and stiffness have been reduced to account for user specified Service Level and/or environmental treatment.
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - SPF	3.50"	2.25"	1.50"	689	1895	641	2592	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	1.50"	782	1274	919	2426	1 1/4" Rim Board

• Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 5" o/c	
Bottom Edge (Lu)	11' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	1 1/4" to 11' 5 3/4"	N/A	16.4	--	--	
1 - Uniform (PSF)	0 to 11' 7" (Front)	5' 6"	12.0	40.0	-	Floor Load
2 - Point (lb)	4' 8" (Front)	N/A	260	-	780	Point coming down from roof
3 - Point (lb)	8' 11" (Front)	N/A	260	-	780	Point coming down from roof
4 - Point (lb)	0 (Front)	N/A	-	621	-	

• Side loads are assumed to not induce cross-grain tension.

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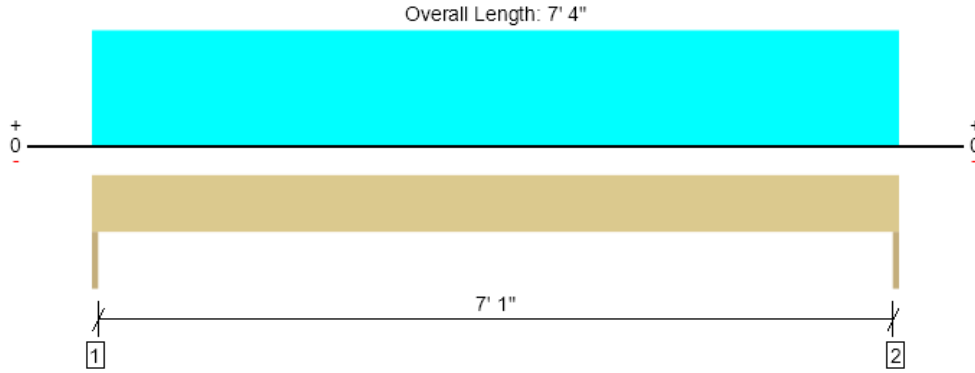
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Ryan Schultz OpeningDesign (773) 425-6456 ryan@openingdesign.com	



11/8/2024 6:33:56 PM UTC
 ForteWEB v3.8, Engine: V8.4.1.24, Data: V8.1.6.3
 File Name: Project

Level, Existing Window Header - Basement - 7ft
3 piece(s) 1 3/4" x 9 1/2" 1.55E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5515 @ 0	5709 (1.50")	Passed (97%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3738 @ 11"	10308	Passed (36%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	9136 @ 3' 8"	15633	Passed (58%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.133 @ 3' 8"	0.244	Passed (L/664)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.198 @ 3' 8"	0.367	Passed (L/443)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 7' 4"
 System : Wall
 Member Type : Header
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	1830	3153	1760	5515	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	1830	3153	1760	5515	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 4" o/c	
Bottom Edge (Lu)	7' 4" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 7' 4"	N/A	15.6	--	--	
1 - Uniform (PSF)	0 to 7' 4"	10' 6"	12.0	40.0	-	Existing - 1st Floor
2 - Uniform (PSF)	0 to 7' 4"	5' 6"	12.0	40.0	-	Existing - 2nd Floor
3 - Uniform (PSF)	0 to 7' 4"	10' 6"	15.0	-	30.0	Existing - Roof
4 - Uniform (PSF)	0 to 7' 4"	5' 6"	12.0	40.0	-	New - 1st Floor
5 - Uniform (PSF)	0 to 7' 4"	5' 6"	12.4	-	30.0	New - Roof

do we need to include these loads, since the windows on the 1st floor are directly below the windows on the basement?



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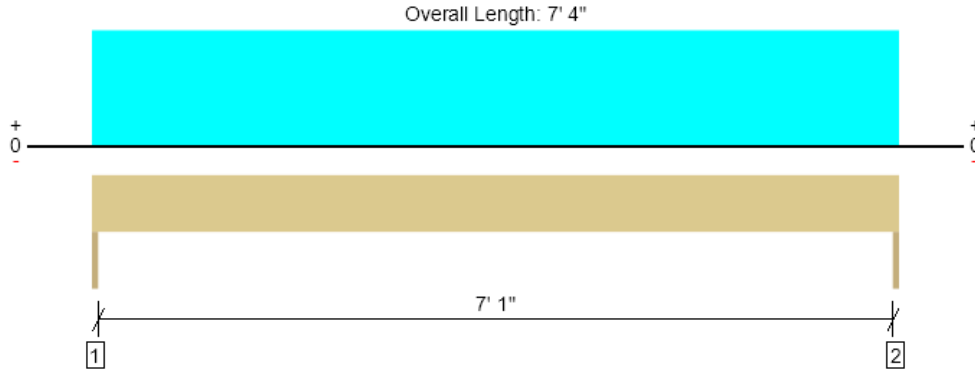
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 File Name: Project

Level, Existing Window Header - First Floor - 7ft
2 piece(s) 1 1/2" x 9 1/2" 1.5E TimberStrand® LSL



Drawing is Conceptual. All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal (typ.).

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3026 @ 0	3263 (1.50")	Passed (93%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	2269 @ 11"	6555	Passed (35%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	5547 @ 3' 8"	9941	Passed (56%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.125 @ 3' 8"	0.244	Passed (L/702)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.197 @ 3' 8"	0.367	Passed (L/447)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

Member Length : 7' 4"
 System : Wall
 Member Type : Header
 Building Use : Residential
 Building Code : IBC 2015
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	1101	807	1760	3026	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	1101	807	1760	3026	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 4" o/c	
Bottom Edge (Lu)	7' 4" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 7' 4"	N/A	8.7	--	--	
1 - Uniform (PSF)	0 to 7' 4"	5' 6"	12.0	40.0	-	Existing - 2nd Floor
2 - Uniform (PSF)	0 to 7' 4"	10' 6"	15.0	-	30.0	Existing - Roof
3 - Uniform (PSF)	0 to 7' 4"	5' 6"	12.4	-	30.0	New - Roof

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